

Preliminary Root Causes Analysis of Failures of the Oroville Dam Gated Spillway

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This Preliminary Root Cause Analysis of the failures of the Oroville Dam gated spillways is based on current publically available photographic and written documentation included and cited at the end of this document.

Design Defects and Flaws

The origins of the gated spillway failures are deeply rooted in pervasive design defects and flaws developed by the California Department of Water Resources (DWR). These design defects and flaws included the following:

1. Spillway base slabs of insufficient thickness for the design hydraulic conditions: 4 to 6 inches thick at minimum points;
2. Spillway base slabs not joined with 'continuous' steel reinforcement to prevent lateral and vertical separations;
3. Spillway base slabs designed without effective water stop barriers embedded in both sides of joints to prevent water intrusion under the base slabs;
4. Spillway base slabs not designed with two layers of continuous steel reinforcement (top and bottom) to provide sufficient flexural strength required for operating conditions; and
5. Spillway base slabs designed with ineffective 'ground' anchors to prevent significant lateral and vertical movements.

Construction Defects and Flaws

The design defects and flaws were propagated by DWR during construction of the spillway. These construction defects and flaws included the following:

1. Failure to excavate the native soils and incompetent rock overlying the competent rock foundation assumed as a basic condition during the spillway design phase, and fill the voids with concrete, and
2. Failure to prevent spreading gravel used as part of the under-slab drainage systems and 'native' soils to form extensive 'blankets' of permeable materials in which water could collect and erode.

¹ Amended to include references and citations to photographs and graphs attached to the Preliminary Root Causes Analysis document.

Maintenance Defects and Flaws

The design and construction defects and flaws were propagated by DWR during maintenance of the spillway. These maintenance defects and flaws included the following:

1. Repeated ineffective repairs made to cracks and joint displacements to prevent water stagnation and cavitation pressure intrusion under the base slabs with subsequent erosion of the spillway subgrade; and
2. Allowing large trees to grow adjacent to the spillway walls whose roots could intrude below the base slabs and into the subgrade drainage pipes resulting in reduced flow and plugging of the drainage pipes.

February 2017 spillway releases

By the time of the February 2017 spillway releases, the gated spillway had become heavily undermined and the subgrade eroded by previous flood releases. The first spillway release completed the undermining of the spillway slabs, allowing water cavitation and stagnation pressures to lift the ‘weak’ slabs and break them into pieces (25, 26).²

After the almost catastrophic water release over the un-surfaced Auxiliary Spillway, the subsequent water releases down the gated spillway propagated the initial spillway breach until spillway releases ceased.

Root Causes Analysis

Currently available information indicates the Root Causes of the gated spillway failures are founded primarily in 'Extrinsic' uncertainties (human and organizational task performance and knowledge development and utilization) developed and propagated by DWR during the gated spillway design, construction, and maintenance activities (1).

A key question that can not be answered at this time is: “why did DWR and the responsible State and Federal regulatory agencies (California Water Commission, Federal Energy Regulatory Commission) allow these Root Causes to develop and persist during the almost 50 year life of the gated spillway?”

One answer that has been offered is that the spillway was designed and constructed according to the ‘Standards of the time.’ While that answer may or may not be factual or true, current evidence indicates the original spillway design and construction does not meet current guidelines and standards (24).

Another answer that has been offered is that the spillway operated for almost 50 years and was subjected to water discharges that exceeded those developed during 2017 without failure. Recent inspections indicated that the spillway was in ‘satisfactory condition’ (5 - 17). The conclusion prior to the February 2017 discharges was the gated spillway consequently was ‘suitable for service.’ The experience prior to the DWR attempt on February 11 to use the Emergency

² References and citations ordered alphabetically at end of this report.

Spillway showed that conclusion was not valid. The gated spillway failed during discharges that were much less than the design conditions.

The author's previous experiences with investigations of failures of public infrastructure systems (e.g. New Orleans hurricane flood protection system during Hurricanes Katrina and Rita) leads to a conclusion that it is likely that the wrong standards and guidelines are being used to re-qualify many critical infrastructure systems for continued service. The majority of these standards and guidelines were originally intended for design, not re-qualification or re-assessment of existing aged infrastructure systems that have experienced 'aging,' 'technological obsolesce,' and increased risk (likelihoods and consequences of major failures) effects. Inappropriate standards and guidelines are being used to re-qualify these infrastructure systems for continued service. The currently available information indicates this is one of the primary Root Causes of the failures of the Orville Dam gated spillway.

References

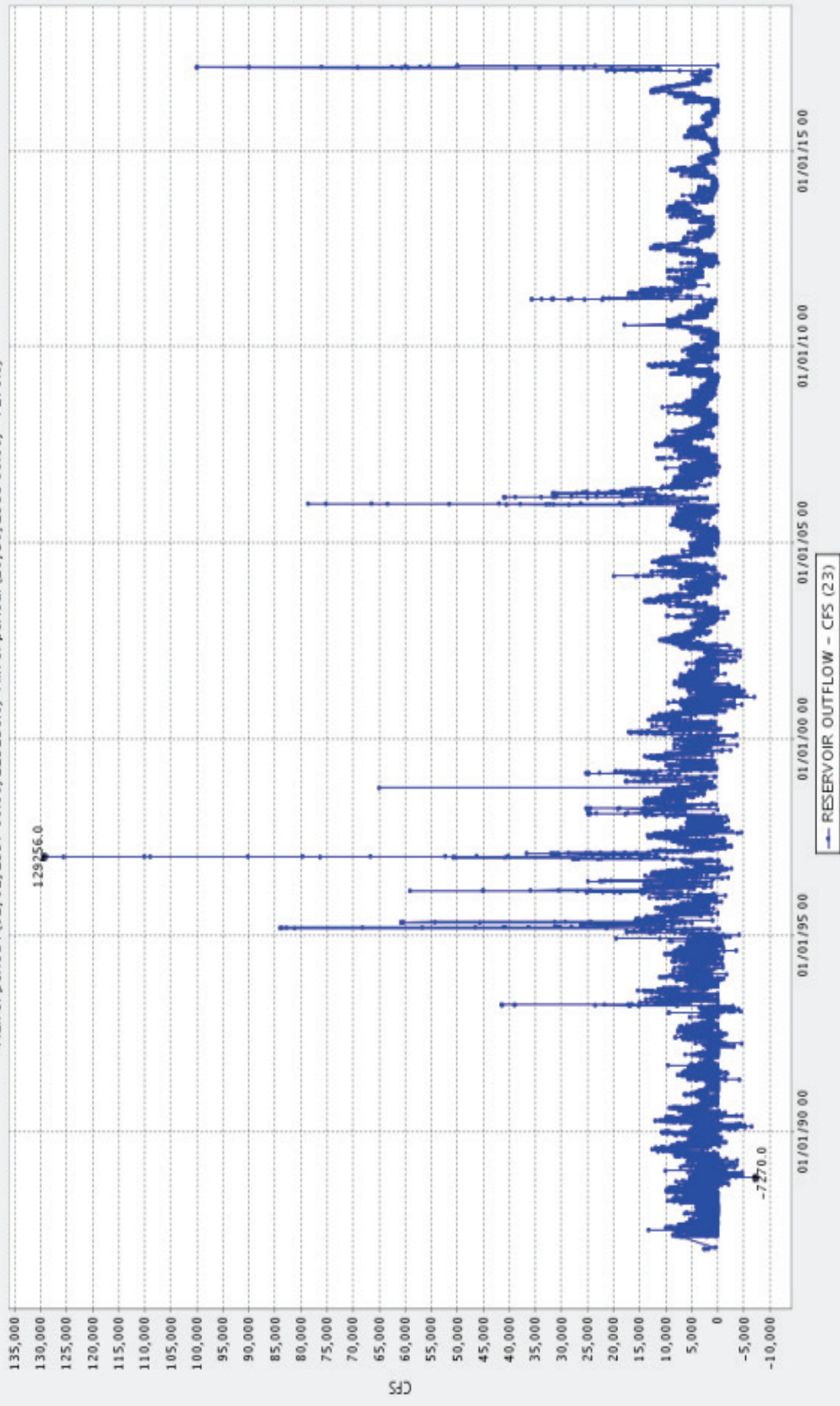
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4. California Department of Water Resources (1965): Oroville Dam Spillway Terminal Structure, Concrete and Details, February.
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Spillway Discharges

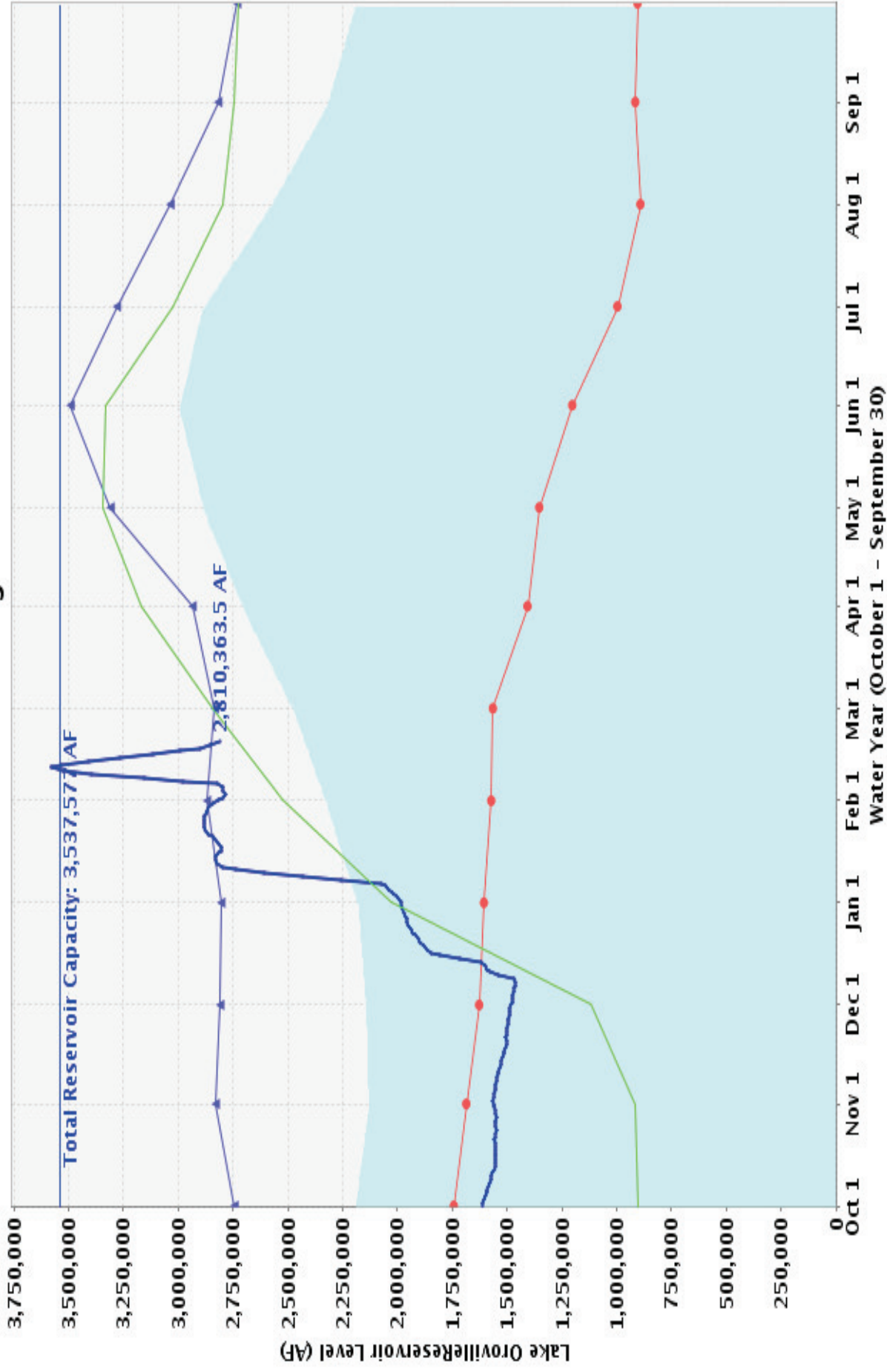
OROVILLE DAM (ORO)

Date from 01/01/1984 through 03/01/2017 07:18 Duration : 12113 days
Max of period : (01/02/1997 00:00, 129256.0) Min of period: (10/30/1988 00:00, -7270.0)



Source: (21)

Lake Oroville Storage Levels

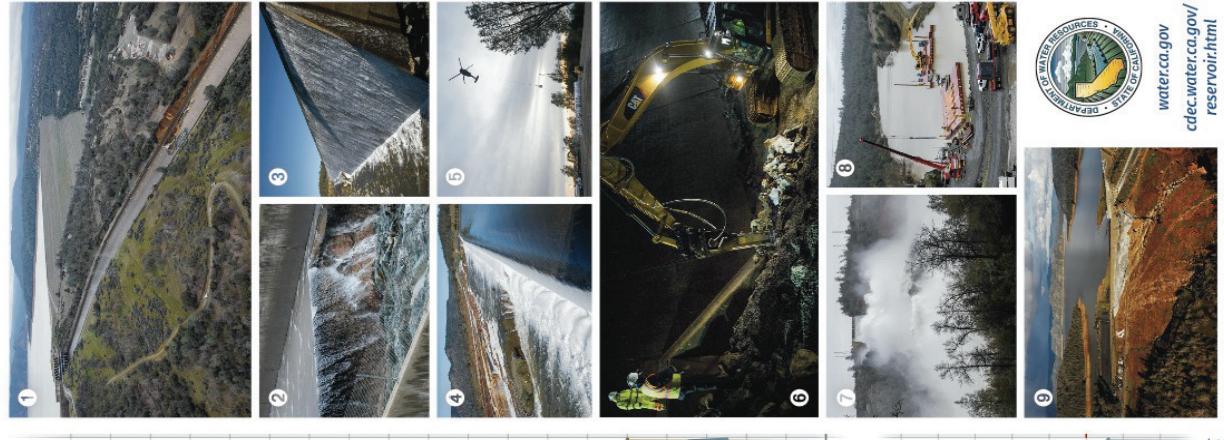
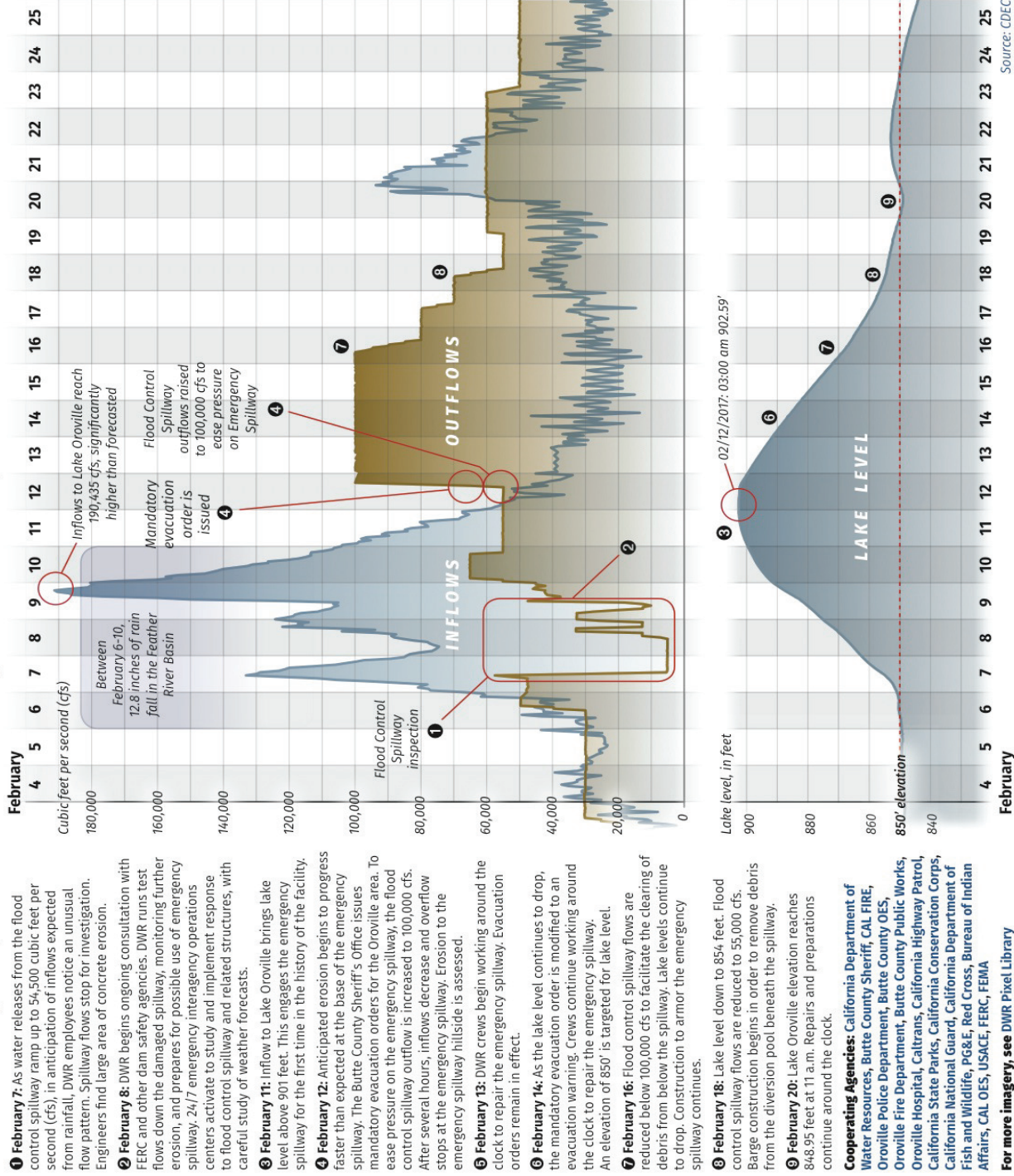


■ Historical Average — Total Reservoir Capacity 3,537,577 AF
— 1977 - 1978
● 1976 - 1977 (dry) ▲ 1982 - 1983 (wet) — 2016 - 2017 (current)

Source: (21)

Lake Oroville Spillway Incident: Timeline of Major Events February 4-25

Oroville Spillway Public Info Line: (530) 538-7826



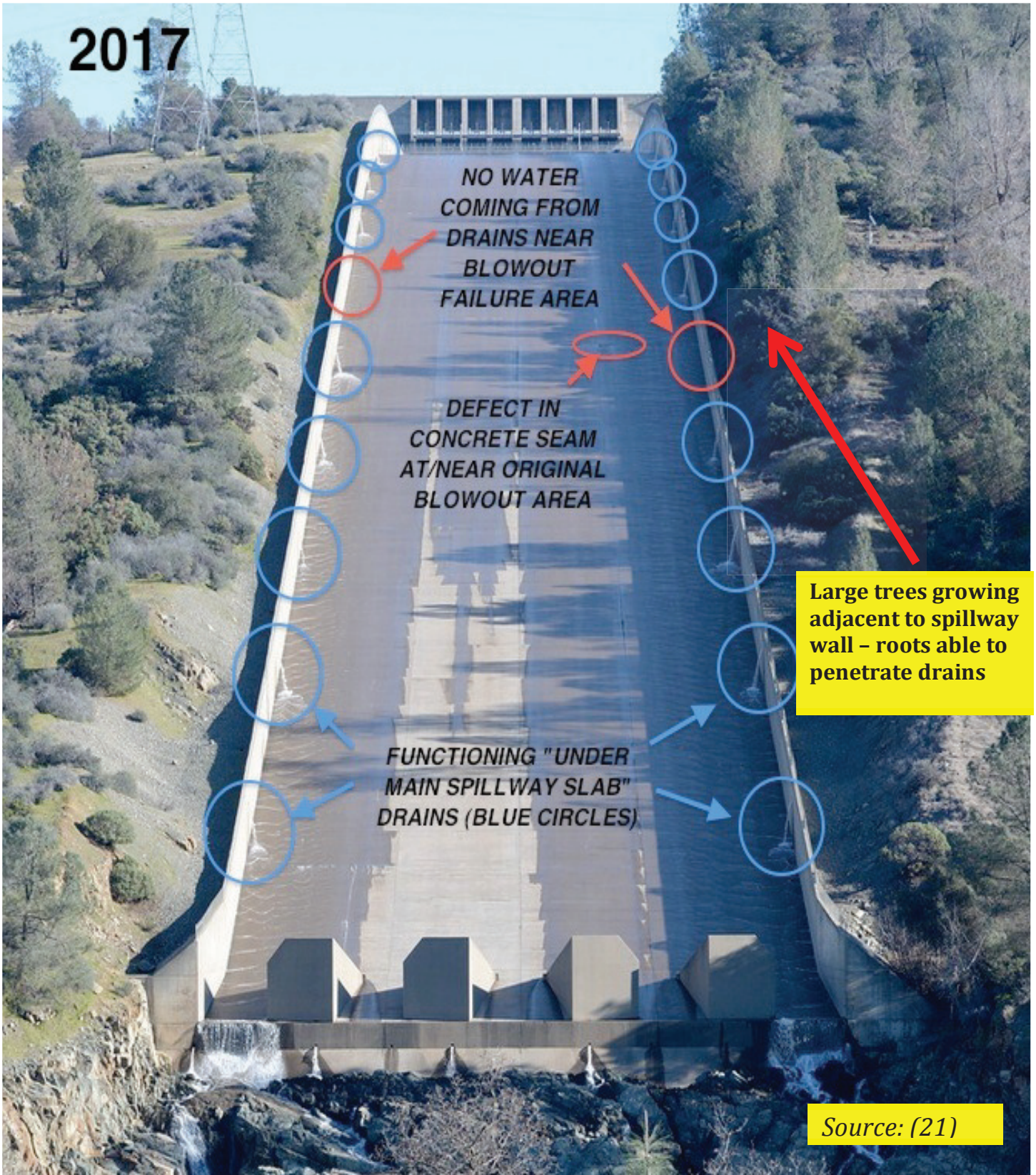


water.ca.gov

cdec.water.ca.gov/reservoir.html

Source: (18)

January 27, 2017



February 7, 2017 - Stage #1



Plume of 'dirty' water - eroded sediments from under spillway

Source: (19)



Exposed rock and erodible sediments under spillway

Source: (19)

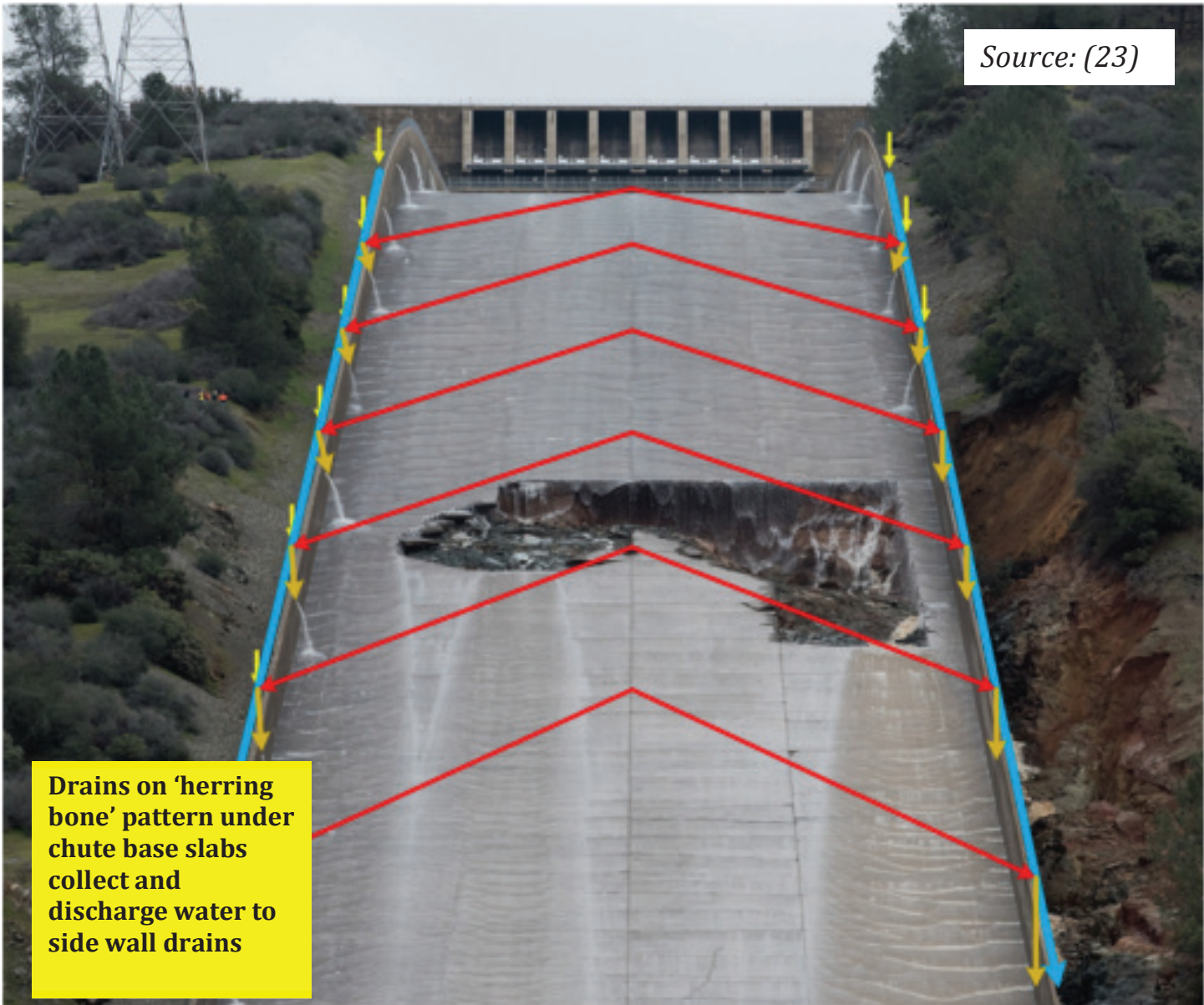


Source: (19)



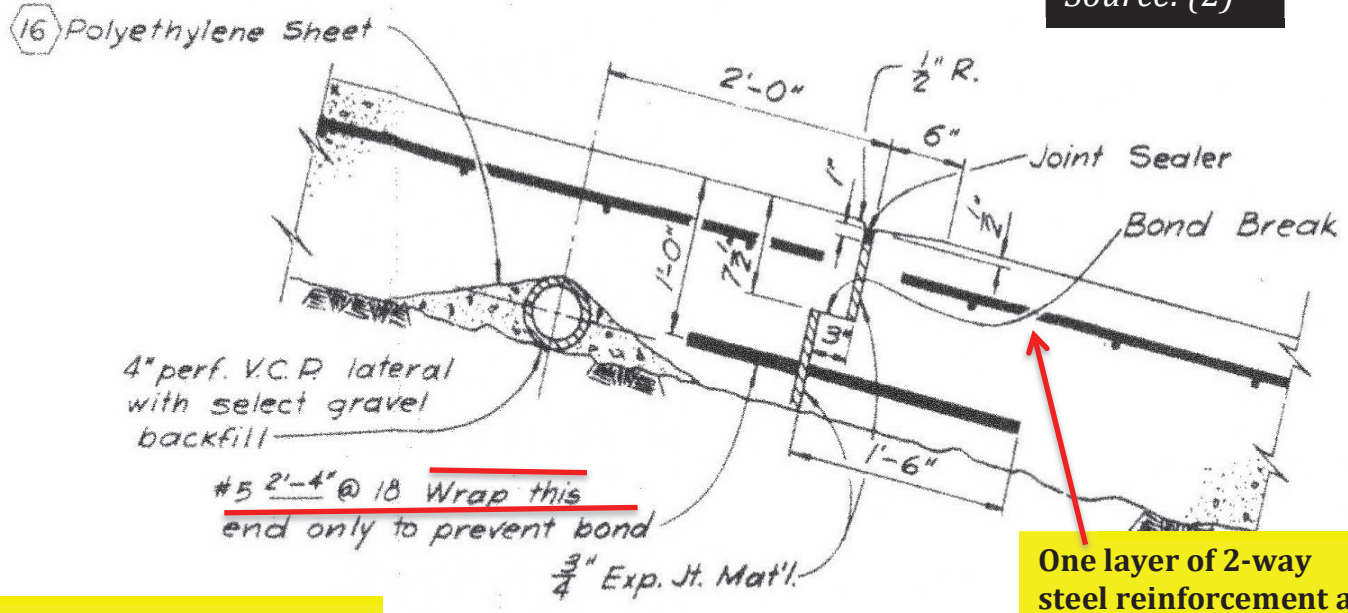
Source: (21)

Source: (23)



Drains on 'herring bone' pattern under chute base slabs collect and discharge water to side wall drains

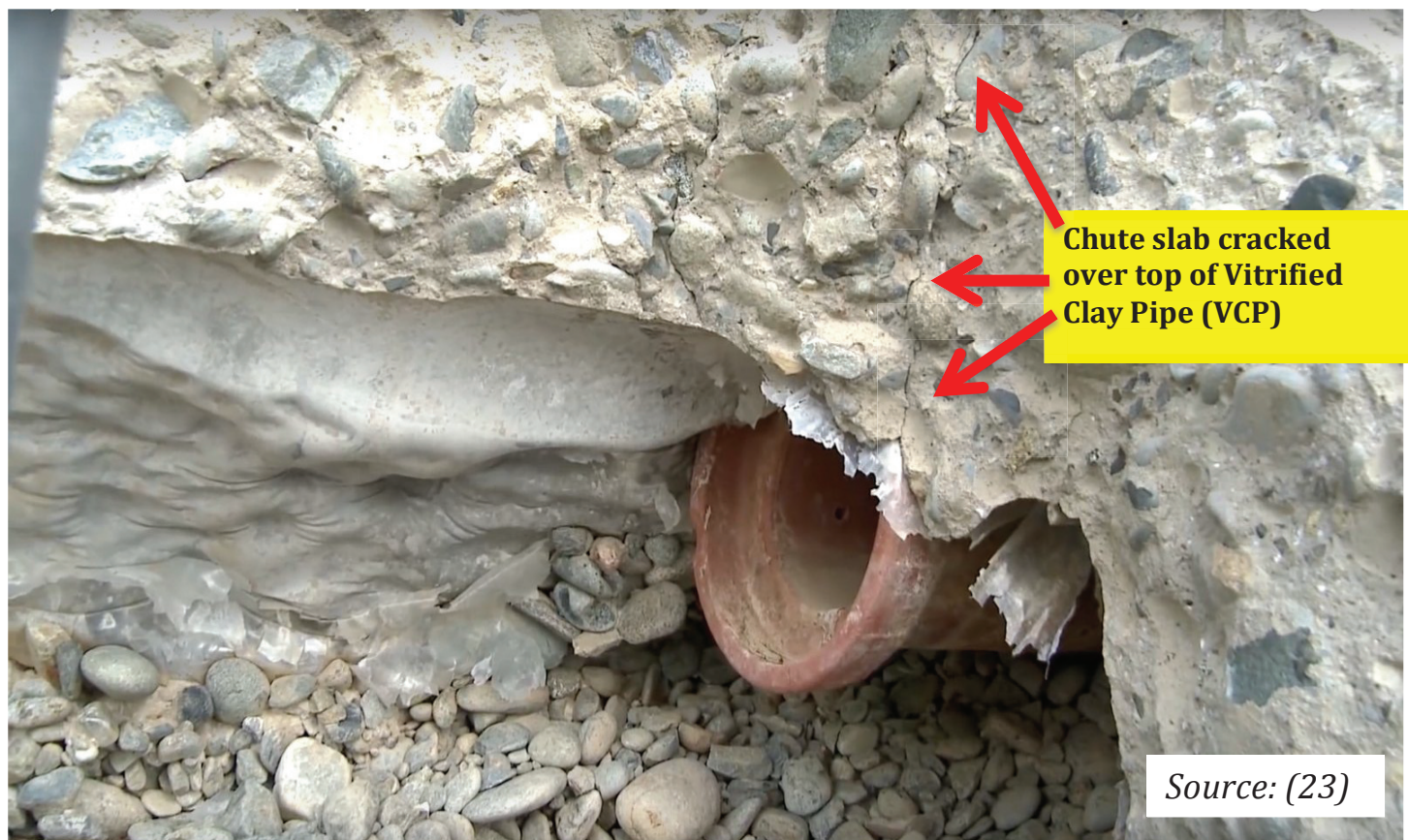
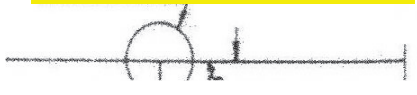
Source: (2)



One layer of 2-way steel reinforcement at top of slab

No 'continuous' steel across joint to prevent slab separation

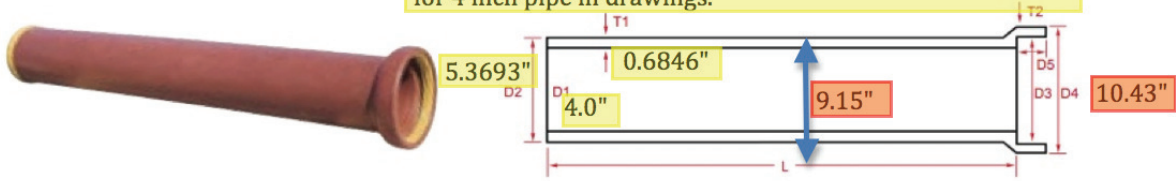
DETAIL A
LATERAL EXPANSION JOINT
Scale: 1" = 1'



Source: (23)

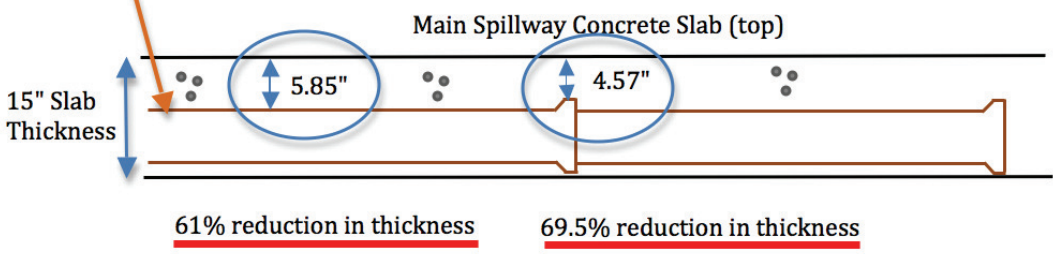
Standard Bell & Spigot

Oroville Design 1"=1' dwg dimension scale converted values for 4 inch pipe in drawings.



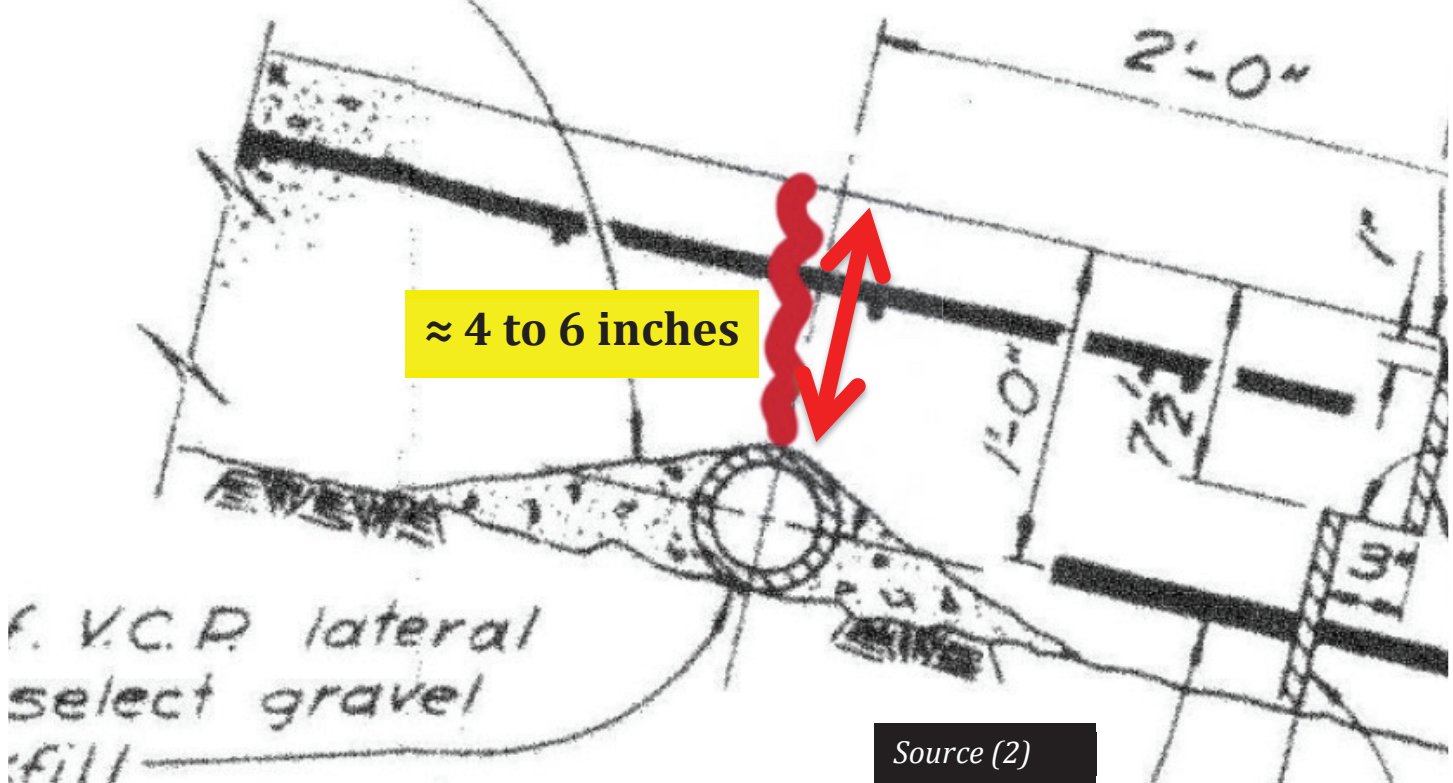
4.0" D1			5.3693" D2			0.6846" T1		
D1	L	Weight plf	D2	D3	D4	D5	T1	T2
4"	1', 2', 4'	11 lb	5.3125"	6.5"	7.625"	1.5"	.6875"	.5"
6"	1', 2', 6'	21 lb	7.87"	9.05"	10.43"	2.33"	7.87"	0.67"
6" D1			7.87" D2			10.43" D4		

Spillway DESIGN Changed to 6" I.D. Drain Pipe: New dimensions should be very close to this table (6" D1, 7.87" D2, 10.43" D4)

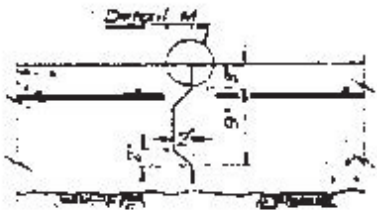


Source (23)

ene Sheet



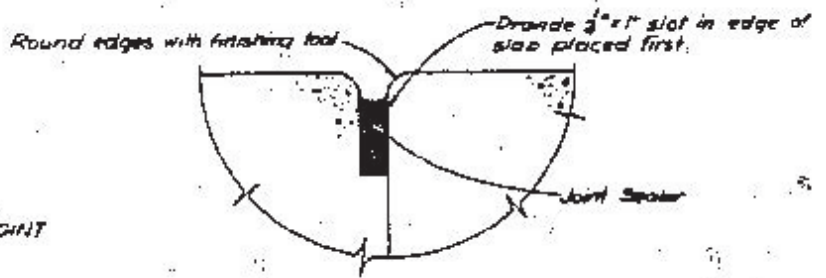
Source (2)



TYPICAL DETAIL
LONGITUDINAL INVERT CONTRACTION JOINT
Scale: 1"=1'

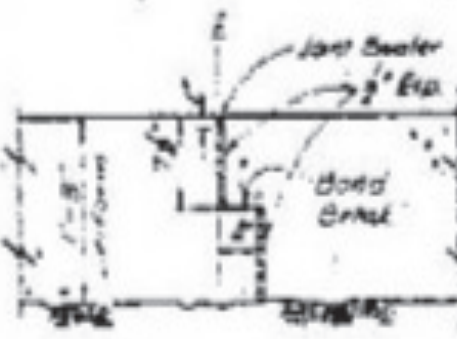
No steel reinforcement
across slab joints to
prevent separation

DETAIL A
LATERAL EXPANSION JOINT
Scale: 1"=1'



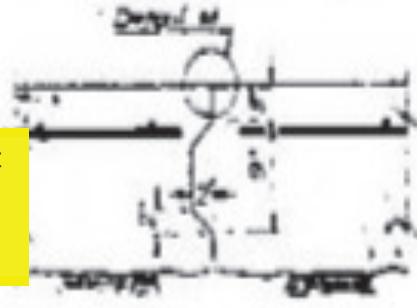
DETAIL M
Scale: 1/2"=1'

A-559-G



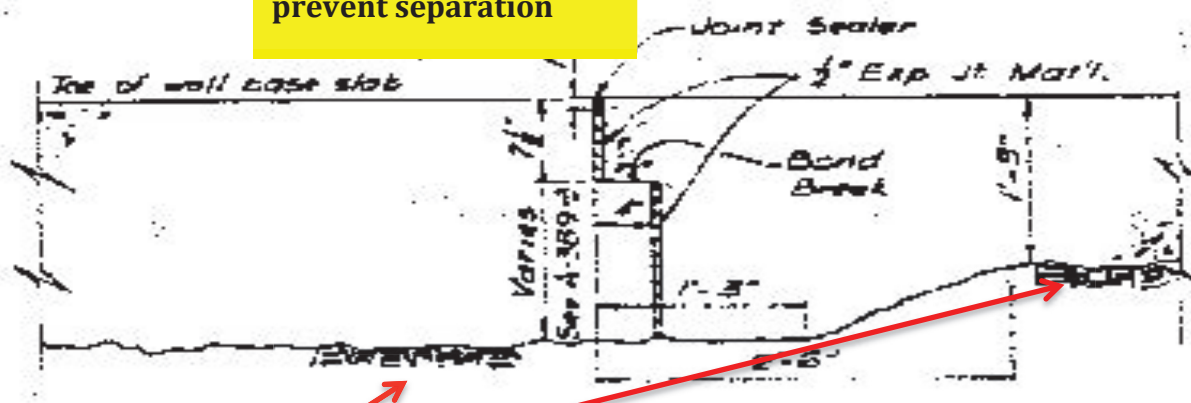
TYPICAL DETAIL
EXPANSION JOINT
Scale: 1"=1'

No steel reinforcement
across slab joints to
prevent separation



TYPICAL DETAIL
LONGITUDINAL INVERT CONTRACTION JOINT
Scale: 1"=1'

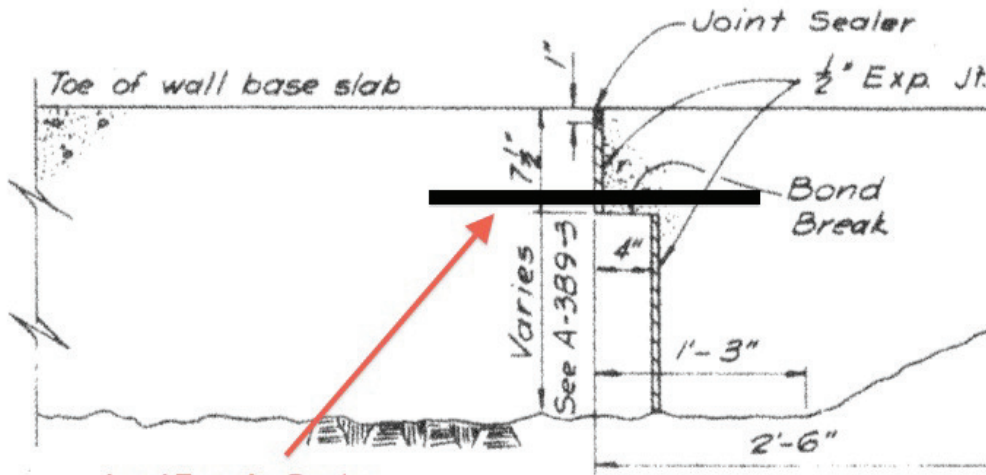
No steel reinforcement
across slab joints to
prevent separation



TYPICAL DETAIL
SLAB EDGE EXPANSION JOINT
Scale: 1"=1'

Chute base slabs
designed to be
constructed on 'rock'

Source (2)



Load Transfer Bar (as constructed - not shown in diagram)

TYPICAL DETAIL
SLAB EDGE EXPANSION JOINT
 Scale: 1" = 1'

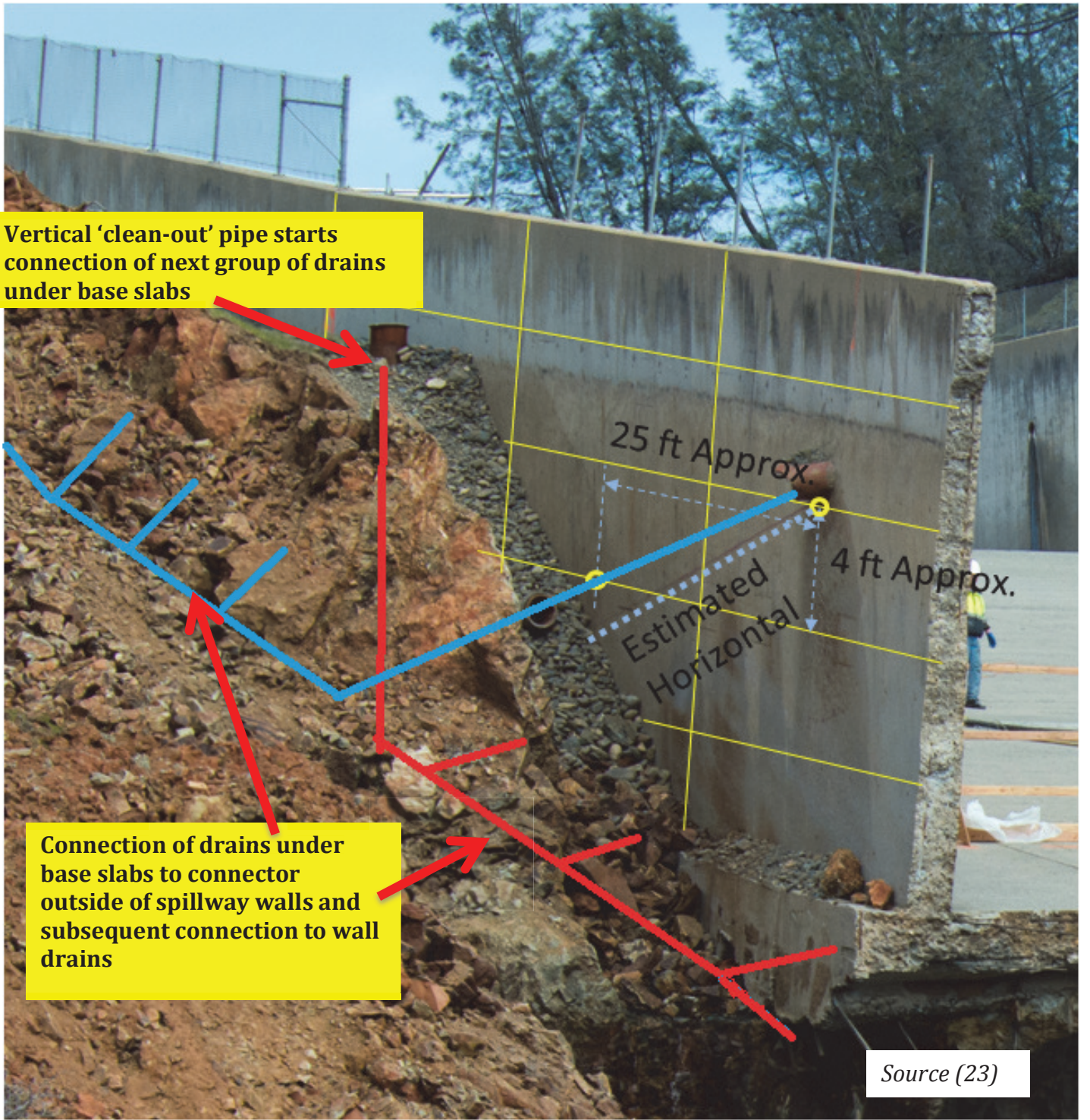
Source (2)



Source (23)



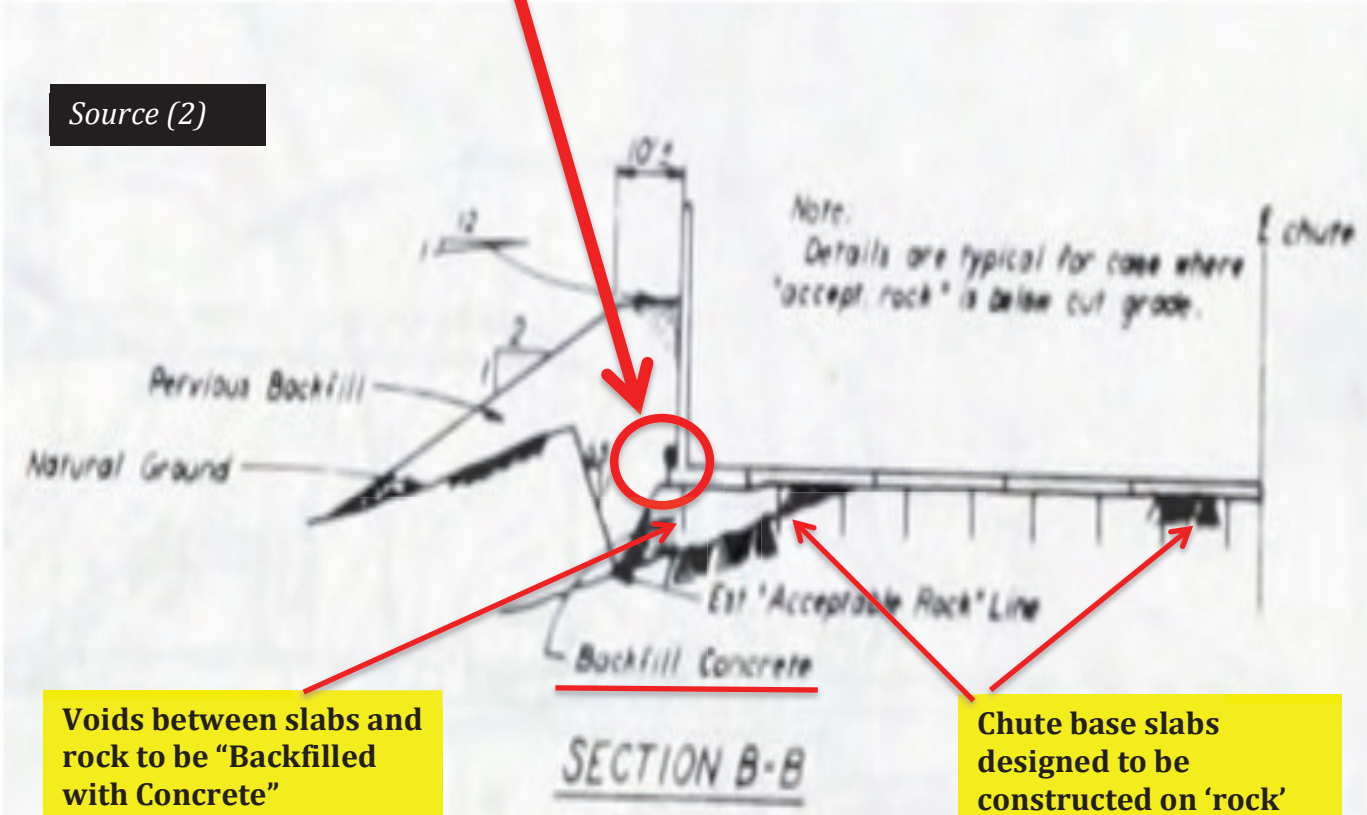
Source (19)





Source (19)

Sidewall drain



Source (2)

Voids between slabs and rock to be "Backfilled with Concrete"

Chute base slabs designed to be constructed on 'rock'

February 9, 2017 - Stage #2

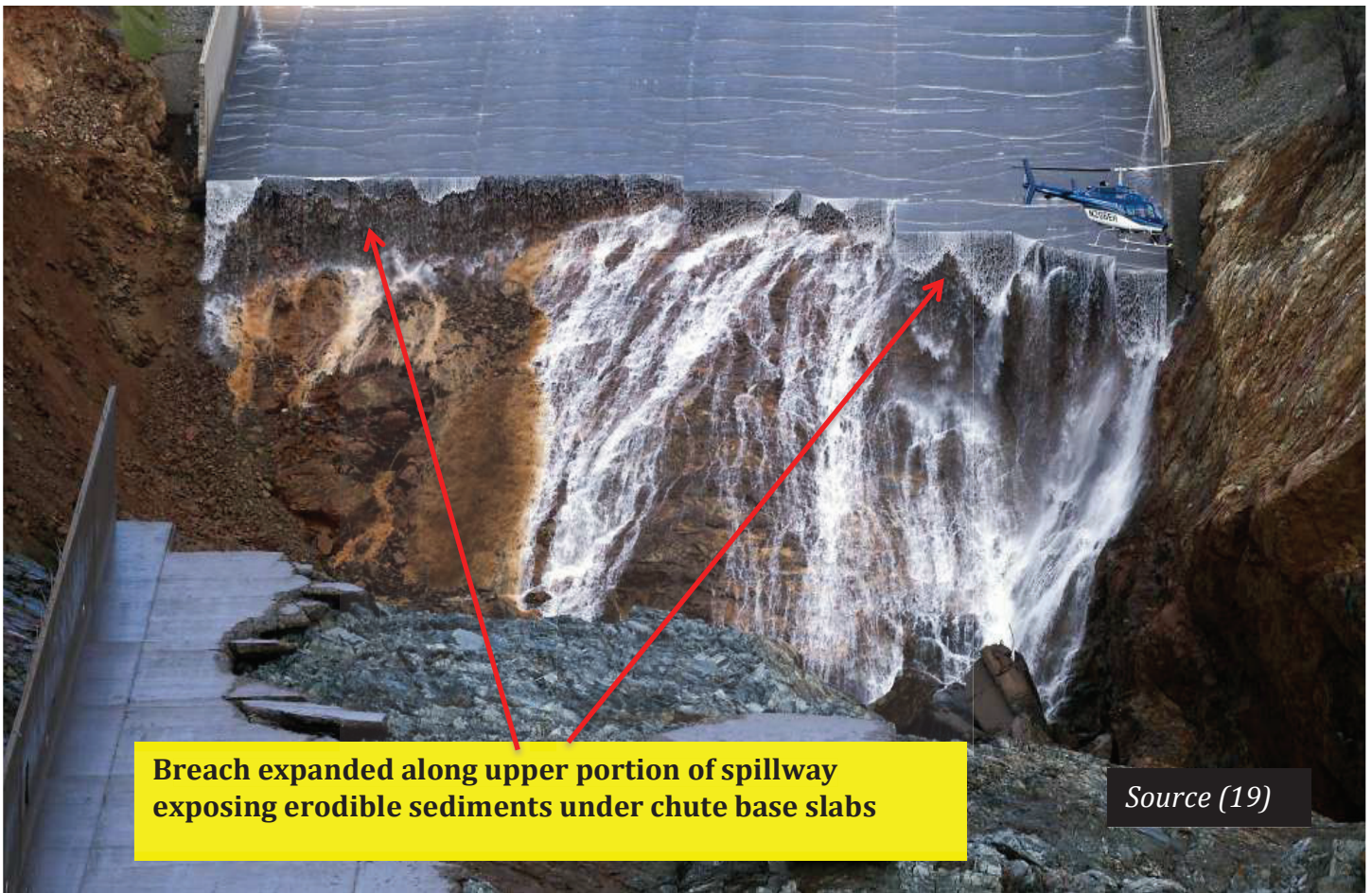






Both sides of spillway chute walls breached and eroding sediment outside of spillway

Source (19)



Breach expanded along upper portion of spillway exposing erodible sediments under chute base slabs

Source (19)





'Incompetent' weathered rock

Source (21)

Stage 3 - February 16, 2017



Water flowing from broken longitudinal drain pipe outside spillway training wall

Source (19)



Source (19)



Source (19)



Source (19)



Source (19)

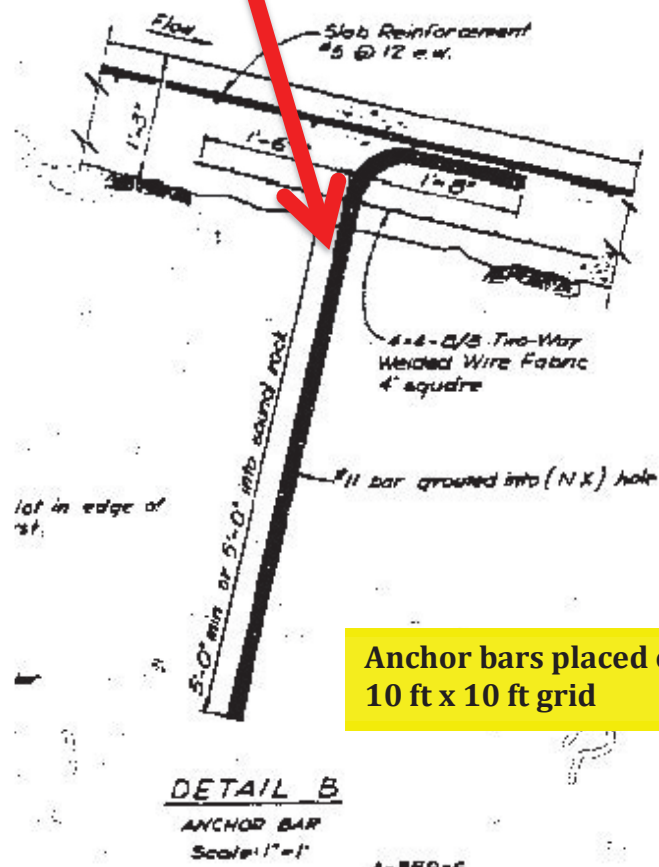


One layer of 2-way steel reinforcement at top of chute slabs

Source (19)



Source (19)



Anchor bars placed on 10 ft x 10 ft grid

Source (2)

Stage #4 – Temporary Repairs to spillway chute





Source (19)



Source (19)



Source (23)

Source (23)

Concrete fractured along
drain pipe emplacement





**Flanged steel pipe
under spillway base
slabs ????**